

Kurdistan Engineers Union

یەکیته ئه‌ندازیاری کوردستان



Using GIS for Determine Peak Flood Discharge of Karam Bast Dam



For Kurdistan Engineering Union

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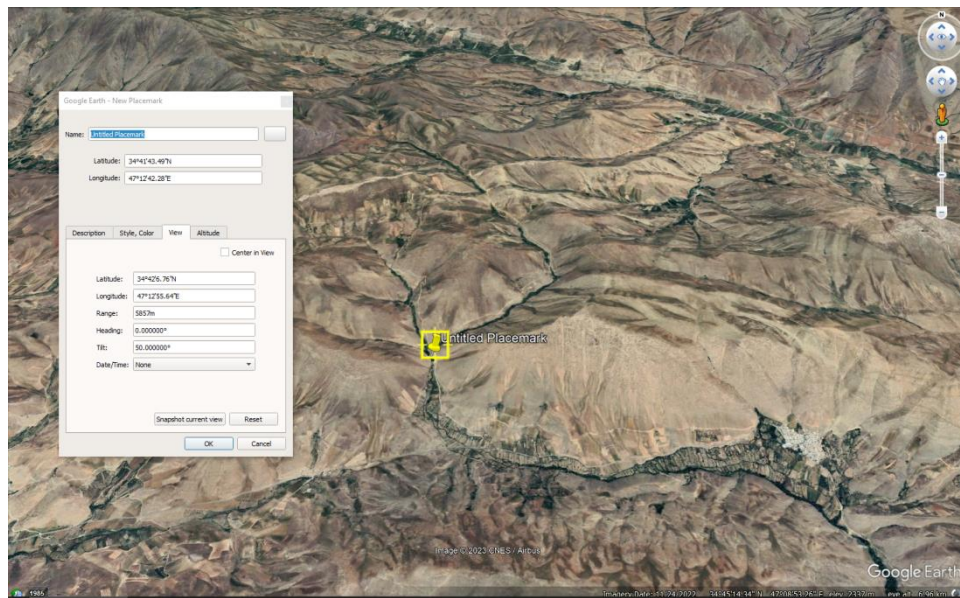
KEU ID: (6474)

Karam Bast Dam:

Karam Bast Dam is located in the northern Kermanshah province that is considered a semi-humid area with cold water and air from a regional point of view. coordinate is located in geographical specifications with length $47^{\circ}12'42.90''E$ and width $34^{\circ}41'42.14''N$, The location of the dam in the UTM system is as follows: Y = 3841500 X = and 702600.

The dam is the source of the agricultural water for the civilian of karam bast and kandula villages.

The medium soil cover of the area is 6.5 mm per hour and the vegetation cover of the area includes 5% medium forest cover, 60% wetland with medium cover and 35% wetland with good cover.



There is a small constant flow in this area that occurred in 1991 with a slight increase in the surface of this waterfall. Below is the photo of the small wire of the area;

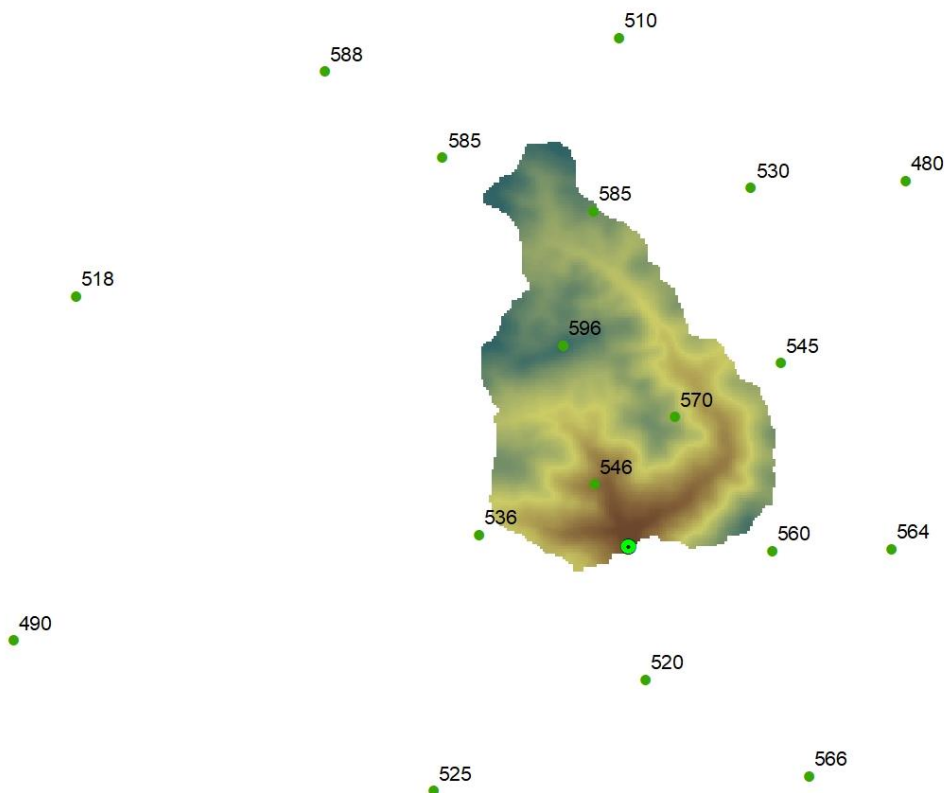


The rainfall gauge:

The rainfall gauge is located surrounding dam that has the bellow information:

**Annual rainfall records are available at each of the gauge
rainfall stations in the region**

No.	X (m)	Y (m)	P (mm)
1	695700	3840400	490
2	696400	3844250	518
3	699180	3846760	588
4	700400	3838720	525
5	700500	3845800	585
6	700910	3841580	536
7	701850	3843700	596
8	702190	3845200	585
9	702200	3842150	546
10	702470	3847140	510
11	702770	3839960	520
12	703100	3842900	570
13	703950	3845460	530
14	705680	3845540	480
15	704190	3841400	560
16	704280	3843500	545
17	704600	3838880	566
18	705520	3841420	564

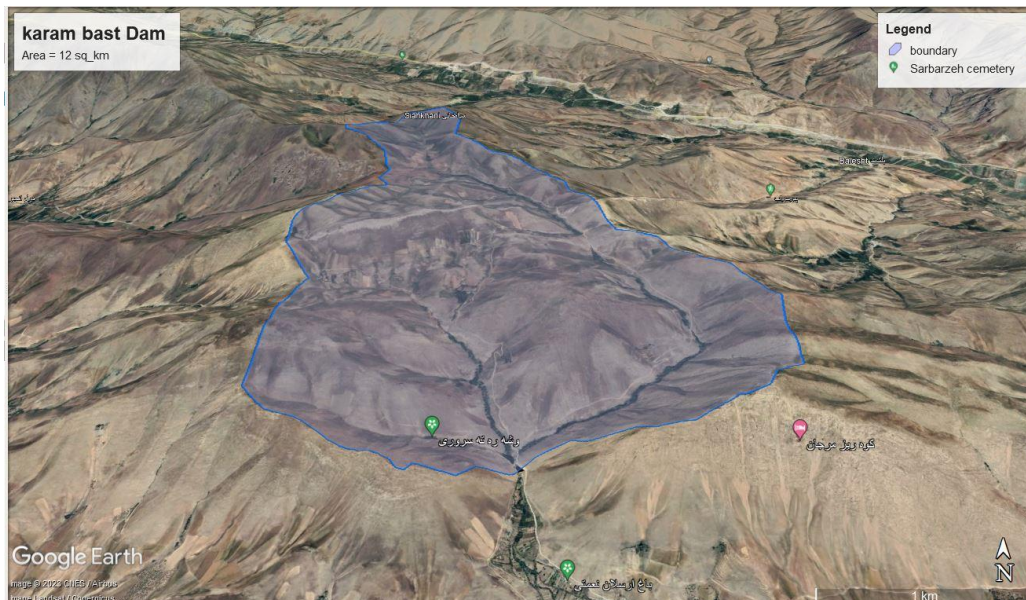
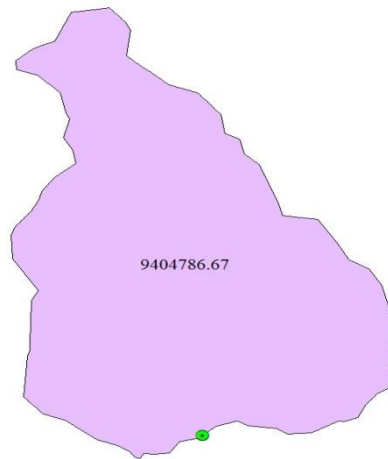


Catchment area:

Is calculated by ArcGIS computer program and exported to location of dam in Google earth map:

Catchment area = $9,404,786.67 \text{ m}^2 = 9.4 \text{ km}^2$

Catchment perimeter = $13,936.30 \text{ m} = 13.936 \text{ km}$



Shape coefficient:

$$\text{Gravelius Coefficient} = 0.28 * 13.936 / (9.4)^{0.5} = 1.27$$

$$L = 1.27 * (9.4)^{0.5} + (((1.27)^2 * 9.4) - (1.2544 * 9.4))^{0.5} / 1.12 = 5.11 \text{ km}$$

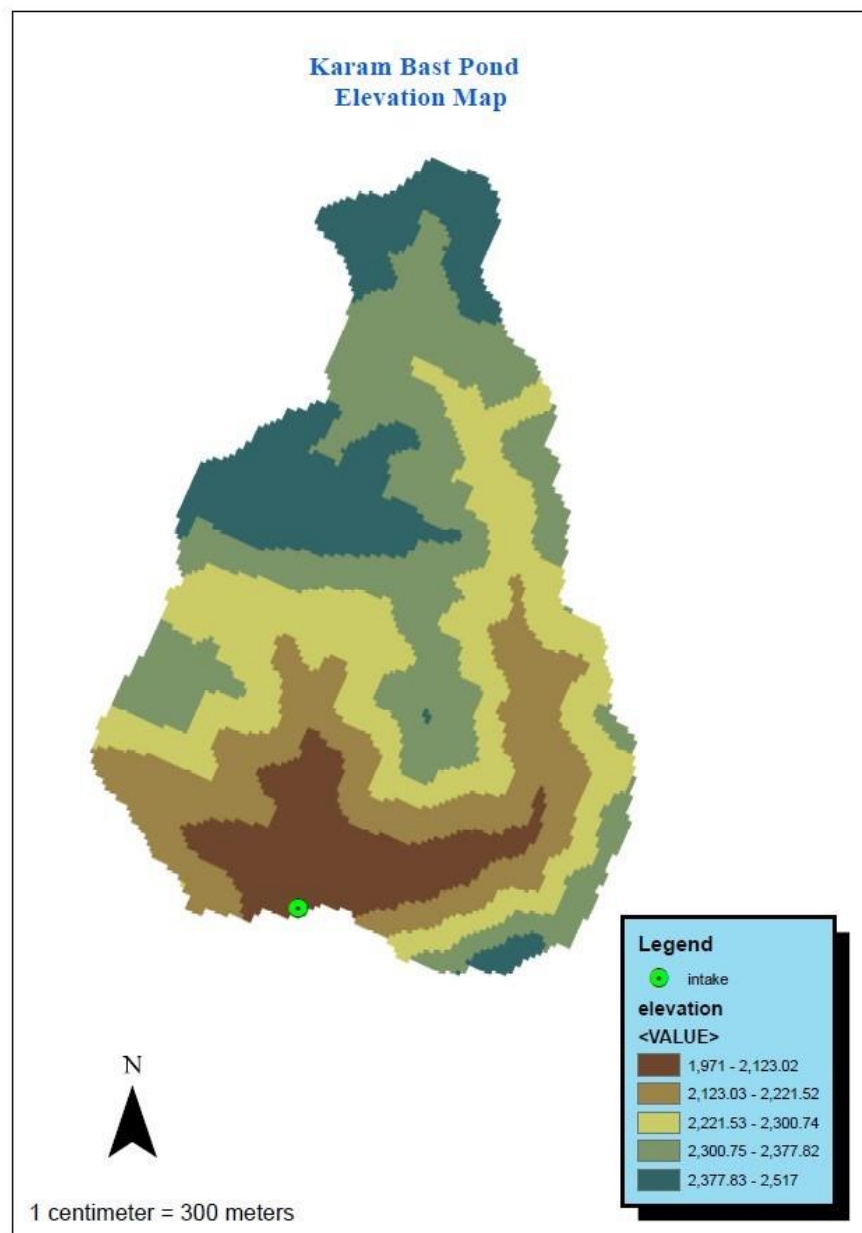
$$B = 1.27 * (9.4)^{0.5} - (((1.27)^2 * 9.4) - (1.2544 * 9.4))^{0.5} / 1.12 = 1.83 \text{ km}$$

Topography of the catchment:

Is calculated by ArcGIS computer program, and has the following results;

Max. elevation of catchment area = 2,517 m

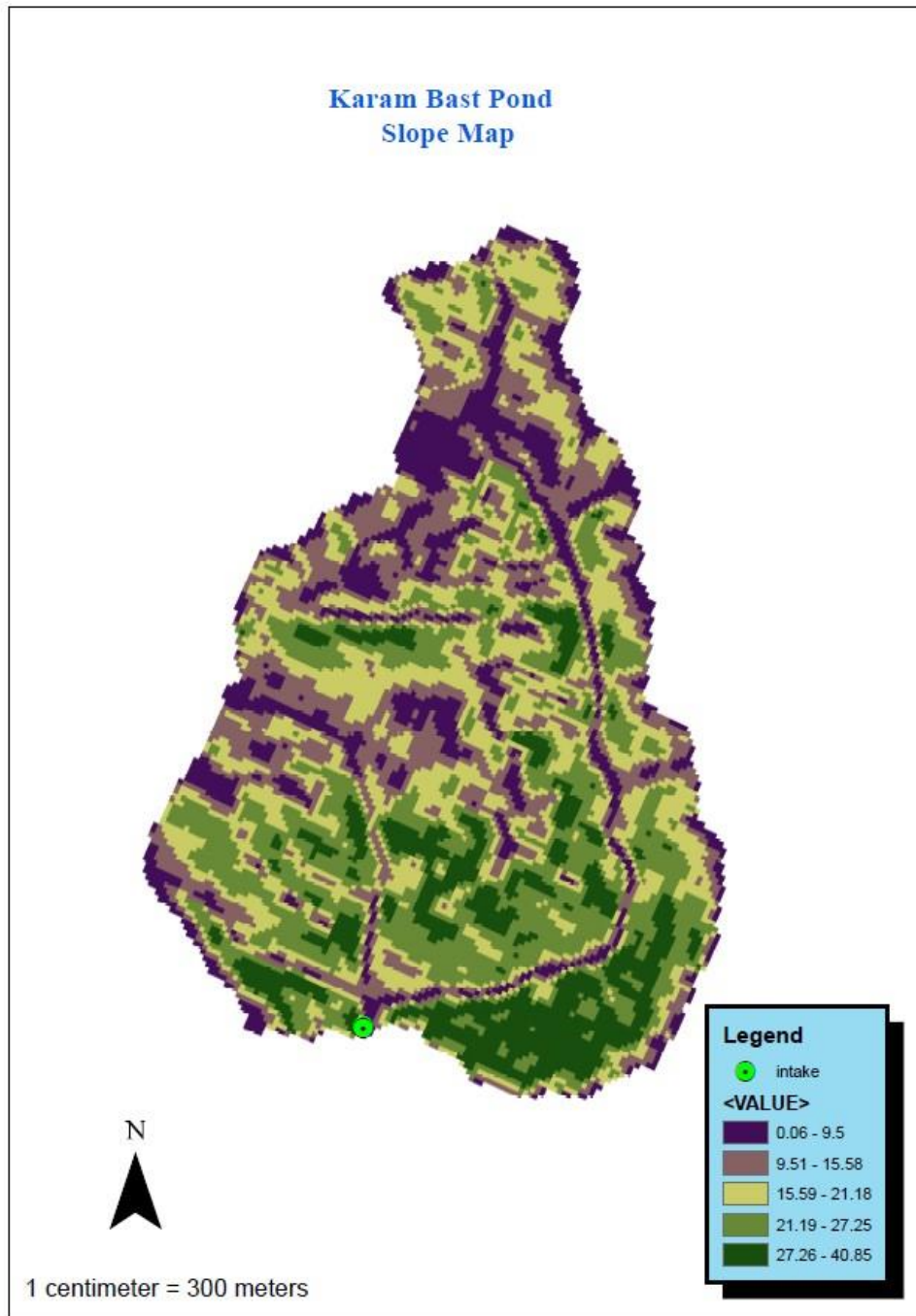
Min. elevation of catchment area = 1,971 m



Slope of the catchment:

Is calculated by ArcGIS computer program, and has the following results;

$$\text{Avg. catchment slope} = (H_{\max} - H_{\min}) / \sqrt{A} = (2.517 - 1.971) / \sqrt{9.4} = 17.81 \text{ m/m}$$

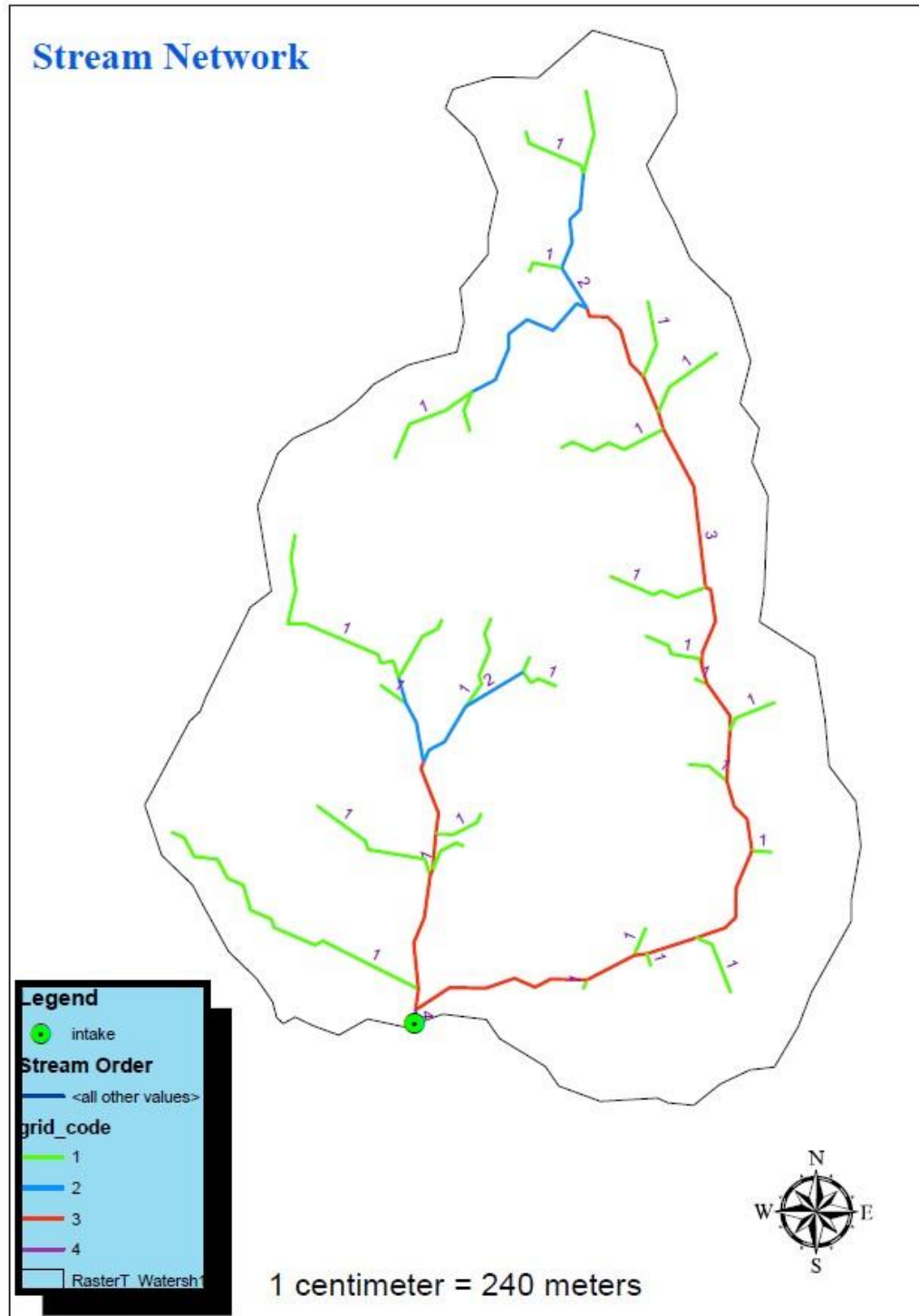


Stream network;

Is calculated by ArcGIS computer program, and has the following results;

The main stream length = 5.68 km

Slope of the main stream line $= (2290 - 1900) / 4.59 = 8.5\%$



Time characteristics of the basin:

$$S = \frac{1000}{CN} - 10$$

$$t_L = \frac{L^{0.8}(S+1)^{0.7}}{(1900)y^{0.5}}$$

$$t_c = 1.66 \times t_L$$

$$t_p = 0.6 \times t_c + \sqrt{t_c}$$

$$Q_p = \frac{0.208 \times A}{t_p}$$

$$T_b = 2.67 \times t_p$$

$$CN = (0.05*60*9.4+0.6*69*9.4+0.35*61*9.4)/9.4 = 65.75$$

$$S = (1000/65.75) - 10 = 5.21 \text{ mm}$$

$$t_{\text{lag}} = (18635.17)^{0.8} (5.21+1)^{0.7} / 1900 * 17.81^{0.5} = 1.17 \text{ hr} = 70.06 \text{ min}$$

$$t_c = 1.67 * 1.17 = 1.95 \text{ hr} = 117.01 \text{ min}$$

$$t_p = 0.6 * 1.95 + \sqrt{1.95} = 2.57 \text{ hr} = 153.99 \text{ min}$$

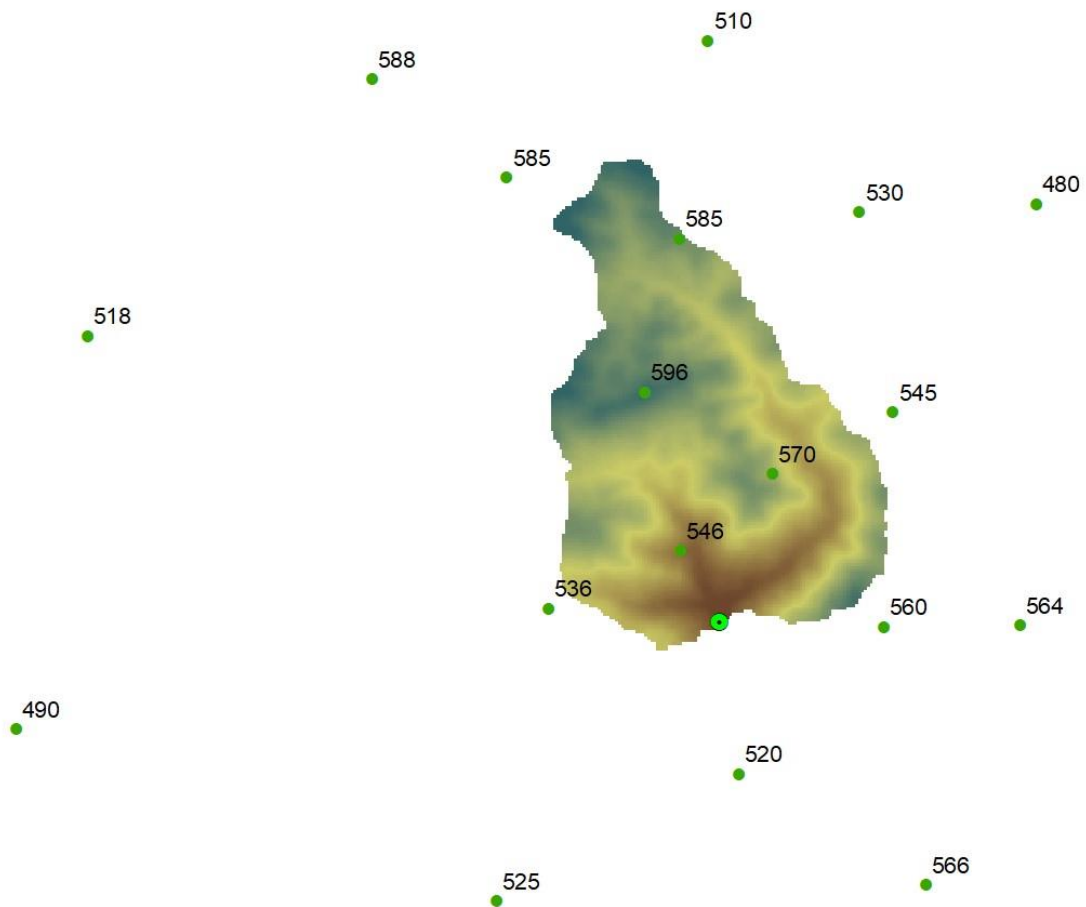
$$Q_p = 0.0208 * 950 / 2.57 = 7.69 \text{ m}^3/\text{s}$$

$$t_b = 2.67 * 2.57 = 6.86 \text{ hr} = 412 \text{ min}$$

Avg. Rainfall Depth over Catchment Area:

1- Arithmetic Method :

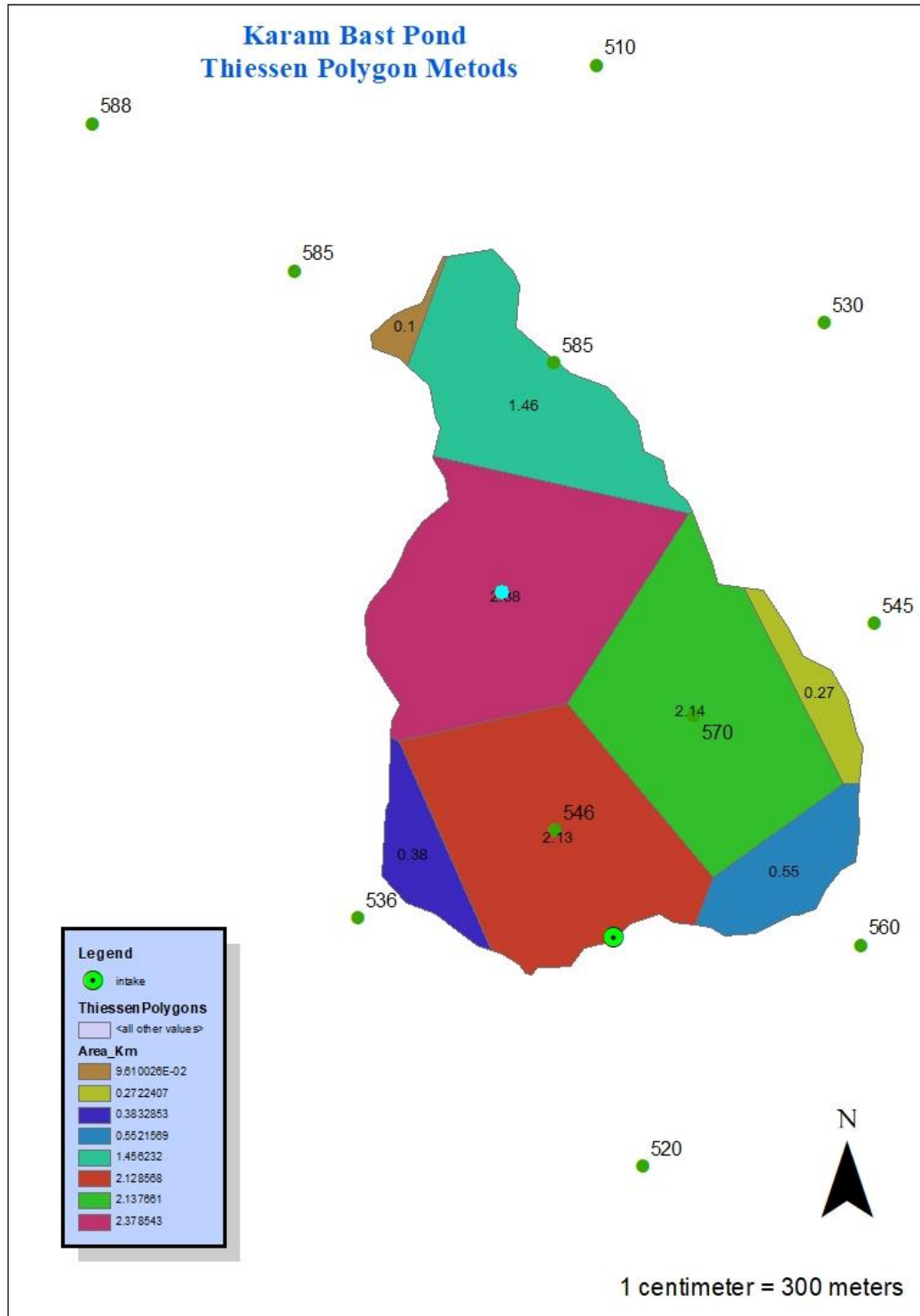
$$P_{\text{avg}} = (585+596+570+546)/4 = 574.25 \text{ mm}$$



2- Thiessen Polygon Method :

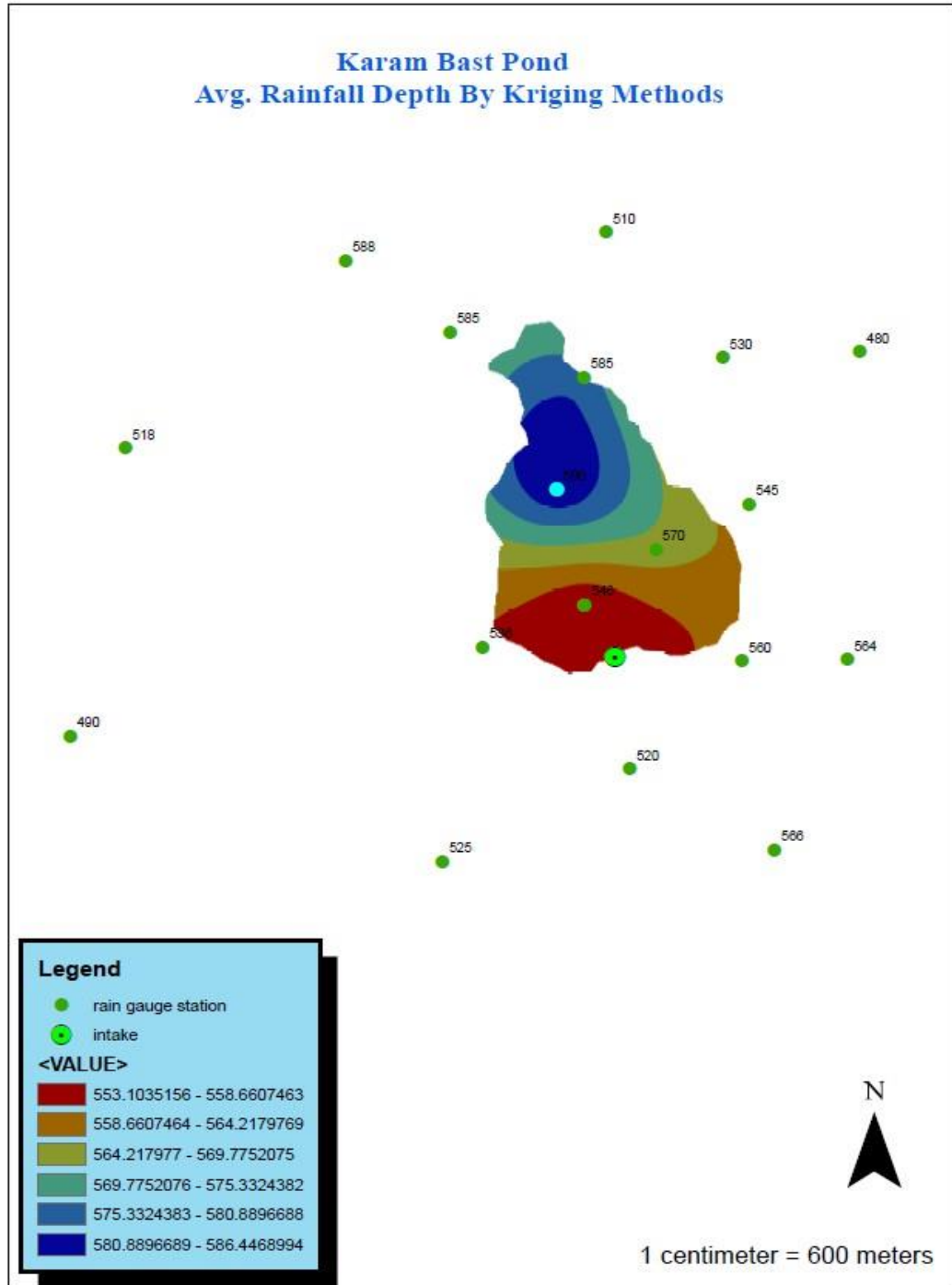
Is calculated by ArcGIS computer program, and has the following results;

$$P_{avg} = (585 * 0.1 + 596 * 2.38 + 585 * 1.46 + 570 * 2.14 + 546 * 2.13 + 545 * 0.27 + 560 * 0.55 + 536 * 0.38) / (585 + 596 + 585 + 570 + 546 + 545 + 560 + 536) = 570.96 \text{ mm}$$

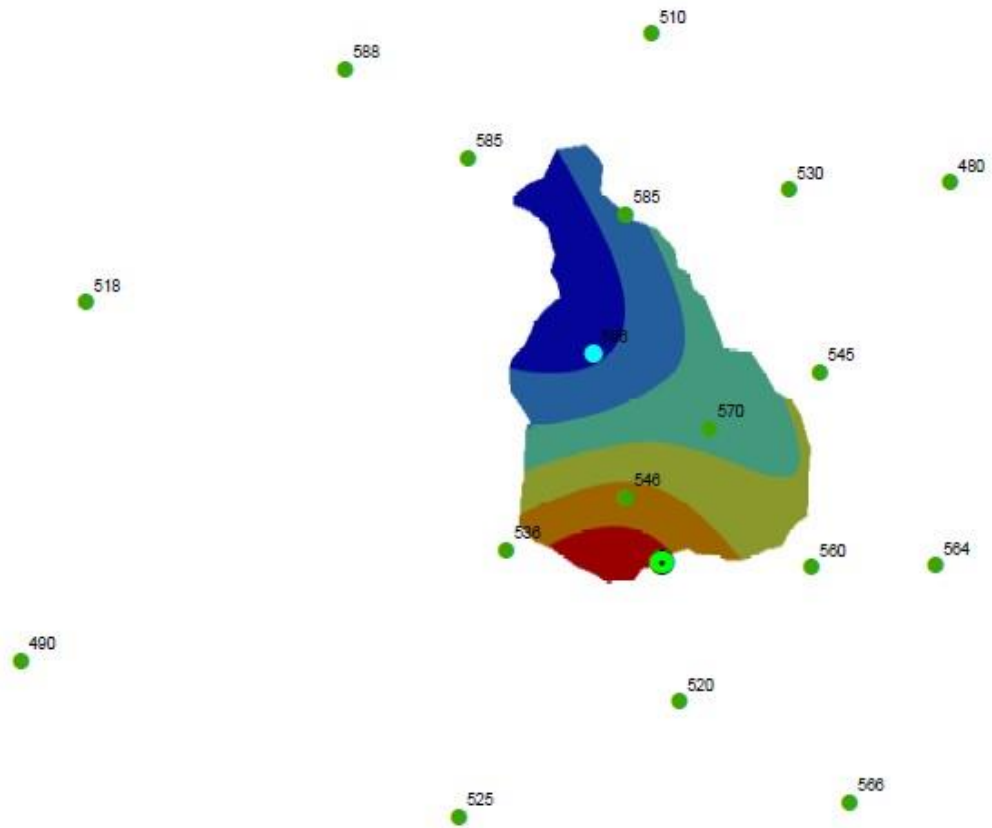


3- Isohyetal Method:

Is calculated by ArcGIS computer program with three methods (kriging, spline and IDW methods), and has the following results;



Karam Bast Pond Avg. Rainfall Depth By Spline Methods



Legend

- rain gauge station
- intake

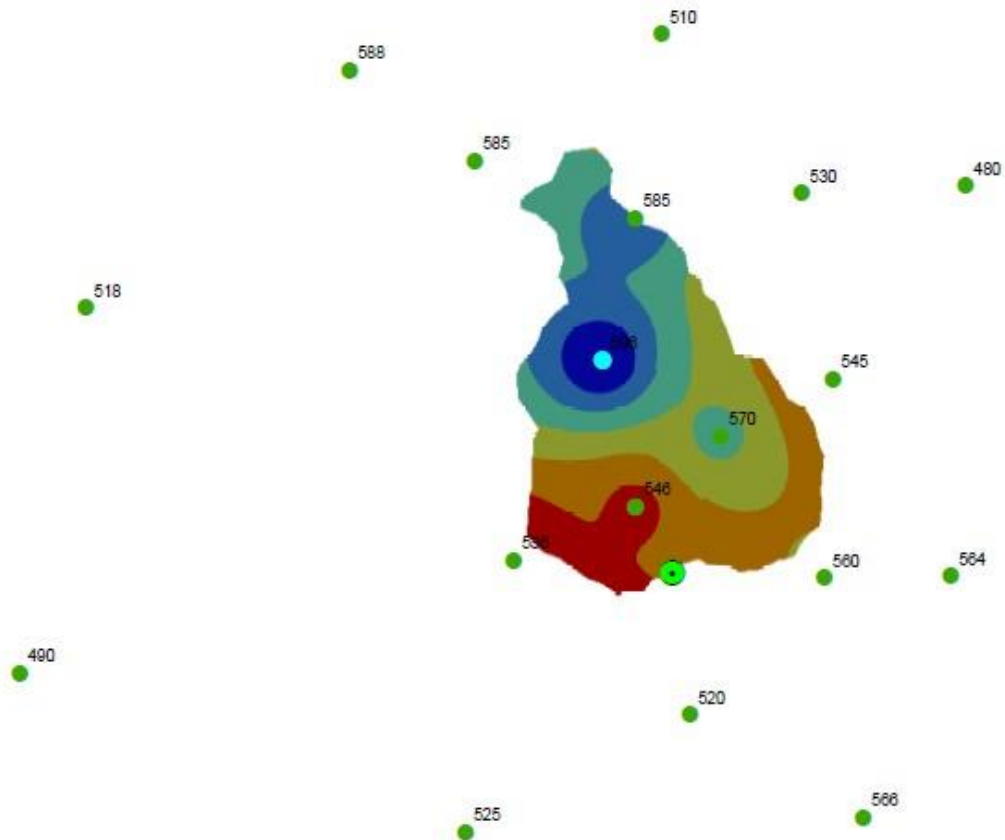
<VALUE>

	522.7651367 - 537.0229187
	537.0229188 - 551.2807007
	551.2807008 - 565.5384827
	565.5384828 - 579.7962646
	579.7962647 - 594.0540466
	594.0540467 - 608.3118286



1 centimeter = 600 meters

Karam Bast Pond Avg. Rainfall Depth By IDW Methods



Legend

- rain gauge station
- intake

<VALUE>

	538.8 - 548.33
	548.34 - 557.87
	557.88 - 567.4
	567.41 - 576.93
	576.94 - 586.47
	586.48 - 596



1 centimeter = 600 meters

Discharge data of the basin for 30 years periods :

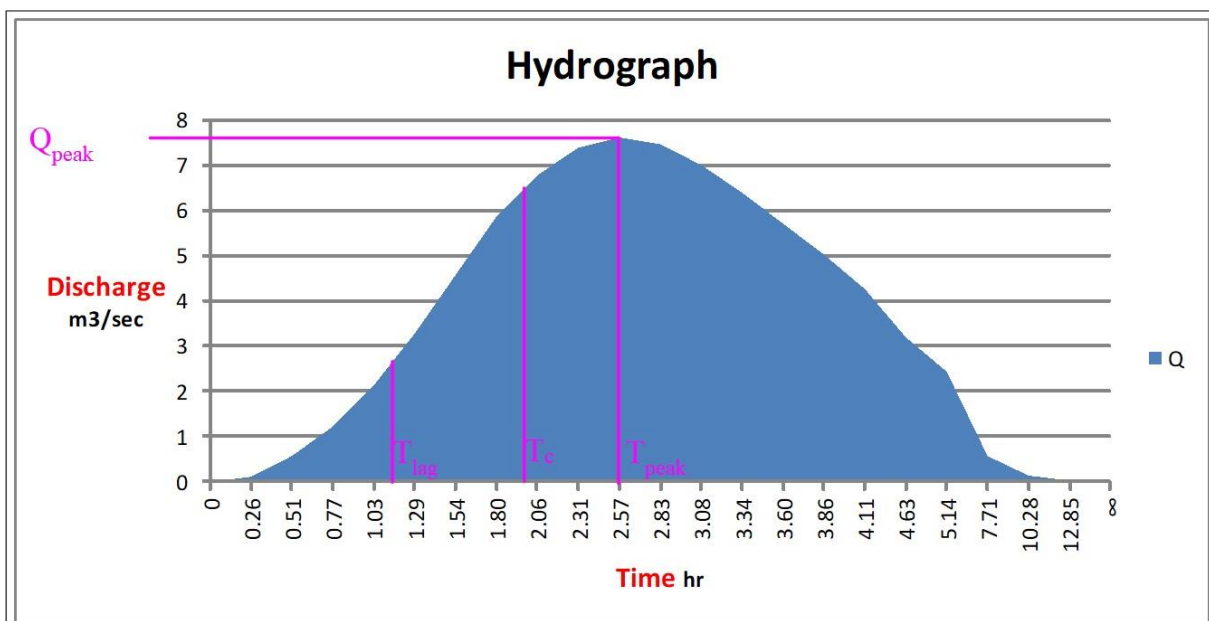
Years	Max. flood discharge
1351-52	0.18
1352-53	1.62
1353-54	1.27
1354-55	0.42
1355-56	0.21
1356-57	2.28
1357-58	1.19
1358-59	2.44
1359-60	1.56
1360-61	0.12
1361-62	1.47
1362-63	0.26
1363-64	0.19
1364-65	0.49
1365-66	2.43
1366-67	1.48
1367-68	0.39
1368-69	0.77
1369-70	0.35
1370-71	1.23
1371-72	0.85
1372-73	0.90
1373-74	1.72
1374-75	0.77
1375-76	0.25
1376-77	1.62
1377-78	0.25
1378-79	0.50
1379-80	0.40
1380-81	0.69

Max. Discharge according $Q_p = \frac{0.208 \times A}{t_p}$

Is equal to 7.69 m³/sec that calculated above with time results of the catchment.

Hydrograph of the catchment by SCS methods;

t/tp	Q/QP	t	Q
0	0	0	0
0.1	0.015	0.26	0.11
0.2	0.075	0.51	0.57
0.3	0.16	0.77	1.22
0.4	0.28	1.03	2.13
0.5	0.43	1.29	3.27
0.6	0.6	1.54	4.57
0.7	0.77	1.80	5.86
0.8	0.89	2.06	6.77
0.9	0.97	2.31	7.38
1	1	2.57	7.61
1.1	0.98	2.83	7.46
1.2	0.92	3.08	7.00
1.3	0.84	3.34	6.39
1.4	0.75	3.60	5.71
1.5	0.66	3.86	5.02
1.6	0.56	4.11	4.26
1.8	0.42	4.63	3.20
2	0.32	5.14	2.44
3	0.075	7.71	0.57
4	0.018	10.28	0.14
5	0.004	12.85	0.03
	0	∞	0



Estimation of the maximum flood discharge with a return period of 25 years;

years	Q m3/s	cumulative	Xi-Mean	(Xi-Mean)^2	(Xi-Mean)^3
1351-52	0.18	0.18	-0.76	0.58	-0.45
1352-53	1.62	1.80	0.68	0.46	0.31
1353-54	1.27	3.07	0.33	0.11	0.03
1354-55	0.42	3.49	-0.52	0.27	-0.14
1355-56	0.21	3.70	-0.73	0.54	-0.40
1356-57	2.28	5.98	1.34	1.78	2.38
1357-58	1.19	7.17	0.25	0.06	0.01
1358-59	2.44	9.61	1.50	2.24	3.35
1359-60	1.56	11.17	0.62	0.38	0.23
1360-61	0.12	11.29	-0.82	0.68	-0.56
1361-62	1.47	12.76	0.53	0.28	0.15
1362-63	0.26	13.02	-0.68	0.47	-0.32
1363-64	0.19	13.21	-0.75	0.57	-0.43
1364-65	0.49	13.70	-0.45	0.21	-0.09
1365-66	2.43	16.13	1.49	2.21	3.28
1366-67	1.48	17.61	0.54	0.29	0.15
1367-68	0.39	18.00	-0.55	0.31	-0.17
1368-69	0.77	18.77	-0.17	0.03	-0.01
1369-70	0.35	19.12	-0.59	0.35	-0.21
1370-71	1.23	20.35	0.29	0.08	0.02
1371-72	0.85	21.20	-0.09	0.01	0.00
1372-73	0.90	22.10	-0.04	0.00	0.00
1373-74	1.72	23.82	0.78	0.60	0.47
1374-75	0.77	24.59	-0.17	0.03	-0.01
1375-76	0.25	24.84	-0.69	0.48	-0.33
1376-77	1.62	26.46	0.68	0.46	0.31
1377-78	0.25	26.71	-0.69	0.48	-0.33
1378-79	0.50	27.21	-0.44	0.20	-0.09
1379-80	0.40	27.61	-0.54	0.30	-0.16
1380-81	0.69	28.30	-0.25	0.06	-0.02
	28.30				

Mean = Sum. Of Discharge / No. years = 28.32/30 = 0.944

Variance = 1.42

Standard Deviation = 1.19

Skewness = 0.23

Cv = 1.26

By Person Type three :

$$X = \bar{x} + K_{25} * S$$

$$K_{25} = 1.797$$

$$X_{25} = 0.944 + (1.19 * 1.797) = 3.09 \text{ m}^3/\text{s}$$

By Gumble;

$$X_{25} = 0.944 + (1.19 * 2.393) = 3.8 \text{ m}^3/\text{s}$$